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Judul Jurnal Ilmiah (Artikel) : Simple Mechanical Beneficiation Method of Coarse Fly Ash with High LOI for Making HVFA Mortar

Penulis Jurnal Ilmiah : Penulis 1, penulis 2, penulis 3

Jumlah penulis : 3 orang

Status Pengusul : penulis pertama / ~~penulis ke ...~~ / ~~penulis korespondensi~~ \*\*

Identitas Jurnal Ilmiah :

- a. Nama Jurnal : Civil Engineering Dimension
- b. Nomore- ISSN : 1979-570X
- c. Vol.,no.,bulan,tahun : Vol. 17 No. 1, July 2015
- d. Penerbit : UK.Petra
- e. DOI artikel : <https://doi.org/10.9744/ced.17.1.38-43>
- f. Alamat web jurnal : <http://ced.petra.ac.id/index.php/civ/article/view/19206>  
<http://repository.petra.ac.id/id/eprint/17076>
- g. Terindeks di : Sinta 2

Kategori Publikasi Jurnal Ilmiah : ☐ Jurnal Ilmiah Internasional / internasional bereputasi \*\*

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Periode II Tahun 2012

Nama Terbitan Berkala Ilmiah  
**CIVIL ENGINEERING DIMENSION**  
**ISSN: 1410-9530**

Penerbit: Universitas Kristen Petra Surabaya

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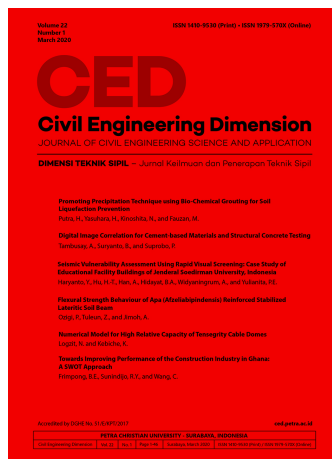
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**Vol. 23 No. 1 (2021): MARCH 2021**



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Published: 2015-03-20

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**CED Indexing & Abstracting**

# Construction Labour Productivity as a Correlate of Project Performance: An Empirical Evidence for Wall Plastering Activity

Odesola, I.A.<sup>1</sup>

**Abstract:** Construction labour productivity has always been believed to be associated with project performance but empirical evidence for this assertion is scarce in literature. This study aims at determining the relationship between construction labour productivity and project performance. Hence, the study evaluates: construction labour productivity, time and cost overruns in wall plastering activity of selected completed public building projects. A survey of 180 purposively sampled public building projects was conducted. Data were collected through project inventory sheet and analysed using ANOVA and Pearson Product Moment Correlation. The results indicate that there is a strong negative correlation between construction labour productivity and cost and time overruns. It also shows that there is no variation in cost and time overruns among the states in the study area. The study therefore, recommends that stakeholders in the construction industry should emphasize the use of productivity improvement strategies on building sites to enhance project performance.

**Keywords:** Construction; labour; project performance; productivity; public building projects; Nigeria.

## Introduction

Productivity is considered as one of the most important factors that affect the success and overall performance of every organization, whether large or small, in today's competitive market [1]. However, Park *et al.* [2] identify construction productivity as a cause of great concern. Veiseth *et al.* [3] and Hewage and Ruwanpura [4] observe that for decades, many researchers have reported the decline in construction productivity. Lawal [5] reports that in Nigeria, construction workers in the public service have almost zero productivity while Kaming *et al.* [6] identify poor productivity of craftsmen as one of the most daunting problems confronting the construction industry especially in developing countries. In view of this, there is a growing and continuous interest in productivity studies all over the world because of its contribution to project cost. Hendrickson and Au [7] state that "good project management in construction must vigorously pursue the efficient utilization of labour, material and equipment and that improvement of labour productivity should be a major and continuous concern of those who are responsible for cost control of constructed facilities".

The reported low productivity in the construction industry is accompanied with its attendant problems of project time and cost overruns, disputes, project abandonment among other problems. According to Aibinu and Jagboro [8], a major criticism facing the Nigerian construction industry is the growing rate of delay in project delivery. Project abandonment is another issue that cannot be ignored on construction sites. Nwachukwu *et al.* [9] maintain that project failure and abandonment are common phenomena in the Nigerian economy.

Studies on cost and time overruns in construction projects have discovered that the construction industry in both developed and developing countries suffers from delays and cost overruns due to labour productivity problems [6,10]. However, despite the many suggestions and opinions in literature concerning the relationship between construction labour productivity and project performance, empirical evidence for these assertions are scarce. The problem of this study is therefore concerned with determining, empirically, the influence of construction labour productivity on the time and cost performance of public building projects. Based on this understanding, this study attempts to investigate the contribution of labour productivity to project performance. The overall aim is to provide empirical evidence to show that labour productivity contributes to project performance with a view to encouraging the adoption of labour productivity improvement techniques in construction project delivery. The objectives are to evaluate construction

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**Note:** Discussion is expected before June, 1<sup>st</sup> 2015, and will be published in the "Civil Engineering Dimension" volume 17, number 2, September 2015.

Received 31 December 2013; revised 29 May 2014; accepted 02 November 2014.

# Characteristics of Motorcycle Ownership and Use of University Students in Malaysian and Indonesian Cities

Putranto, L.S.<sup>1\*</sup>, Prasetijo, J.<sup>2</sup>, and Setyarini, N.P.S.E.<sup>1</sup>

**Abstract:** Motorcycle ownership and use increased rapidly in Indonesian cities in recent years. People could not cope with severe congestion due to unsatisfactory public transport and uncontrolled land use development. This led to motorcycle use for almost any trip. However, in Malaysia motorcycles were mainly used for local short distance travel. In this paper the characteristics of motorcycle ownership and use of university students in Malaysian and Indonesian cities were discussed. A total of 398 university students in eight cities were asked to fill the questionnaires. They consist of general questions regarding their socio-economic background and travel habit along with 25 perceptual questions regarding affordability/attractiveness of owning motorcycle and practicability/safety of motorcycle use. A variance based structural equation modelling called partial least square-path modelling (PLS-PM) was used for analysis. The results show that indicators explaining affordability and acceptability were exactly the same in Penang and combination of seven cities in Indonesia.

**Keywords:** Motorcycle ownership; motorcycle use; PLS-PM.

## Introduction

In developing countries it is common that road and rail based infrastructure could not compete with rapid development of the cities. Cities in South East Asian Countries have long history of high motorcycle ownership and use to face such condition. Vietnam cities have suffered from very high proportion of motorcycle in general traffic (as high as 90% in their main cities) for many years. It has just happened in Indonesia in the last five years. In Indonesian cities, public was so desperate with severe congestion due to unsatisfactory public transport and uncontrolled land use development. Therefore, they tried to find their own way to maintain “acceptable” travel time by using motorcycle. This led to motorcycle use for almost any purpose and wide range of trip length in Indonesian cities. This was not the case in Malaysia, where motorcycles were mainly used for short distance casual travel in the neighbourhood area such as in the university area. This was found in aggregate level research (Malaysian state based research and Indonesian city based research) [1]. This paper is aimed to observe specifically student perception in motorcycle ownership and use in Indonesian and Malaysian cities in disaggregate (individual) level. Students were identified as one of important group of owners and users of motorcycles.

The reason for selecting students in Medan, Palembang, Bandung, Surabaya, Denpasar, Mataram, Makassar, and Penang was to follow up previous research [1] and availability of research partner to conduct data collection.

## Previous Studies

Putranto and Tantama [2] have conducted 24 hours traffic count in Bandung, Surabaya, and Mataram in Indonesia. In each city, two four lanes-two ways divided roads (4/2 D) and two six lanes-two ways divided (6/2 D) road were observed in a normal working day. However, in Mataram no six lanes-two ways road was eligible for observation. Statistical tests showed that percentage of motorcycle in each city was significantly higher than in the Indonesia Highway Capacity Manual (IHCM) [3] reference value, whilst percentage of light vehicle and heavy vehicle in each city were significantly lower than in IHCM [3] reference values. Putranto and Setyarini [4] found similar results in Medan, Makassar, Denpasar, and Palembang.

The following literature reviews are used to justify the indicators of latent variables explaining motorcycle ownership and motorcycle use dimensions:

According to Putranto [5]:

- In a non-car owning household, the higher the number of household members, the higher the number of motorcycles.
- Surrounding the universities, the higher the number of students (number of household members aged >16 years old) the higher the number of motorcycles.

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<sup>2</sup> Civil Engineering Department, Universiti of Tun Hussein Onn, Beg Berkunci 101 Parit Raja, Batu Pahat, Johor West 86400, MALAYSIA.\* Corresponding author; e-mail: lexy\_putranto@yahoo.co.id

**Note:** Discussion is expected before June, 1<sup>st</sup> 2015, and will be published in the “Civil Engineering Dimension” volume 17, number 2, September 2015.

Received 01 December 2013; revised 20 October 2014; accepted 13 December 2014.

## Simple Mechanical Beneficiation Method of Coarse Fly Ash with High LOI for Making HVFA Mortar

Penulis 1, Penulis 2, Penulis 3

**Abstract:** This study focusses on the effect of milling of fly ash obtained from four different sources on the properties of high volume fly ash (HVFA) mortar. Two fly ash samples with low loss-on-ignition (LOI) were taken from a coal-fired power plant, while the other two with high LOIs were obtained from a textile factory and from a paper mill, respectively. Milling was performed using a rod mill at a certain period of time. The workability of HVFA mortar with constant water to cementitious ratio was controlled by adjusting the superplasticizer content. The results show that the specific gravity of fly ash increases after milling. Utilizing milled fly ash ends up with significant strength increase of HVFA mortar, especially those utilizing high LOI fly ash. This shows that milling is an excellent fly ash beneficiation technique, especially on the one with high LOI value.

**Keywords:** Fly ash; HVFA; high LOI; mechanical beneficiation; milling.

### Introduction

Fly ash utilization as partial replacement for cement in making concrete often hindered due to the presence of high amount of unburnt carbon, which is indicated by its high loss-on-ignition (LOI) content. ASTM C618 limits the carbon content not more than 6% to prevent discoloration, poor air entrainment and segregation of fresh concrete mixture [1].

Fly ash properties may vary significantly. It may depend on the type of industry and the process involved in generating this waste material. Power plants generally produce better quality fly ash with low LOI value, due to better control on the burning process, than other industries.

Good quality fly ash with low LOI is desirable by the ready mix industries. Its usage reduces the cost of concrete production, and at the same time it also results in good performance of concrete, such as more workable, higher strength, better durability, and lower shrinkage. However, there are large quantities of low quality fly ash with high LOI values available [2,3]. Without any proper treatment, due to its poor performance, this kind of fly ash is hardly used in concrete production,

and ends up with the disposal of the fly ash to landfill. There has been a growing and persistent need to foster the development of efficient recycling method for coal fly ash [2].

Beneficiation of fly ash has been studied in recent years and several methods have been proposed to separate good quality cementitious material from the combustible one, such as separation [2,4], wet milling and acid leaching [5], thermal treatment in fluidized bed reactors [6], ultrasonic sieving and triboelectrostatic separation [1,7], grinding in eccentric vibratory mill [8,9], and reburning [10]. However, most of the methods normally involve complex processes.

Mechanical activation of fly ash through a laboratory-scale ball-milling has been proved to be efficient to improve the reactivity of fly ash as raw material for geopolymer cured under ambient temperature [11, 12]. To enable the wider application of the beneficiation method, especially on the coarse and high LOI fly ash, a simpler beneficiation method should be developed. This study aims to evaluate the mechanical activation of coarse and high LOI fly ash by grinding it using a simple rod mill, to improve its quality and to make it more desirable to be used as cementitious material, especially in producing high volume fly ash (HVFA) mortar.

### Materials and Experimental Program

#### Materials

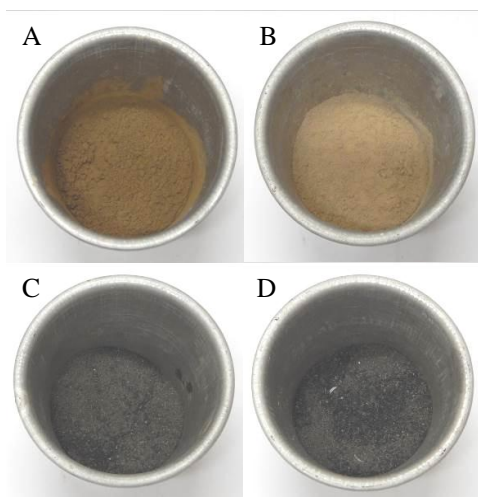
Four different fly ash samples were utilized for the experiment. Fly ash samples coded A and B were obtained from a coal-fired power plant in Paiton, East Java, Indonesia, taken at different time and from different units. Fly ash from this power plant is

**Note:** Discussion is expected before June, 1<sup>st</sup> 2015, and will be published in the "Civil Engineering Dimension" volume 17, number 2, September 2015.

Received 21 February 2015; revised 26 February 2015; accepted 27 February 2015.



well known for its good quality, and is widely used by local ready mix industries. The other two fly ash samples were obtained from a textile factory (C) and from a paper mill (D), both located in East Java, Indonesia. Fly ash A and B have brown and light brown color, while fly ash C and D show grey-blackish color (Figure 1).



**Figure 1.** Four Different Fly Ash Samples

Chemical composition of fly ash was determined by X-ray Fluorescence (XRF) analysis and the results are presented in Table 1. The LOI values of the fly ash show that the quality of the fly ash varies and can be categorized into two types, i.e. the ones with low LOIs (fly ash A and B) and the ones with high LOIs (fly ash C and D). ASTM C618 sets the maximum limit of LOI of not more than 6% for fly ash to be used as cementitious material. High LOI content reflects the high amount of unburnt carbon content in the fly ash, which plays important role in the adsorption capacity of the material [13]. Fly ash B also possesses high CaO content of more than 10%, by mass, and thus can be considered as type C fly ash [14].

### Mechanical Activation

To perform the mechanical activation of the fly ash, a simple rod mill has been constructed. It consists of a steel drum, 600 mm in diameter and 900 mm in length, and filled with 80 kg of 12 mm diameter and 800 mm long steel rods. The drum is rotated about 40 rotation-per-minute. The optimum grinding capacity of the rod mill is between 15-20 kg (see Figure 2).



**Figure 2.** Rod Mill and Steel Rods for Grinding the Fly Ash

From the previous study [15], it was apparent that grinding cementitious materials, in this case calcined Sidoarjo volcanic mud, using this simple equipment increases its reactivity by increasing the specific surface area (SSA) and reducing the particle size of the powder material. With calcined Sidoarjo volcanic mud, it was found that eight hours milling time was the most optimum milling time, and thus the same period of milling time was chosen for this experiment.

Table 2 shows the change of particle size and the specific surface area (SSA) of fly ash due to milling. Milling time 0 hour indicates the original properties of the fly ash before milling, while milling time 8-hour shows the properties of the fly ash after 8 hours milling. It is shown that fly ash A and B are already very fine in its original condition, compared to fly ash C and D. Most of the particles of fly ash A and B are less than 40  $\mu\text{m}$  in size. On the other hand, more than 50% of particles of fly ash C and D, by mass, are bigger than 40  $\mu\text{m}$ . Milling or grinding for eight hours significantly reduced the particle size of fly ash C and D, as it can be seen from the values of D90. D90 denotes the size whereby 90% of the particles, by mass, are smaller than that. However, for the fine fly ash A and B, the change is not as significant as in the case of the coarse ones. The same tendencies can be seen from the change in the values of specific

**Table 1.** Chemical Composition of Fly Ash as Measured by XRF (% by mass)

Fly ash	SiO <sub>2</sub>	Al <sub>2</sub> O <sub>3</sub>	Fe <sub>2</sub> O <sub>3</sub>	TiO <sub>2</sub>	CaO	MgO	Cr <sub>2</sub> O <sub>3</sub>	K <sub>2</sub> O	Na <sub>2</sub> O	SO <sub>3</sub>	Mn <sub>2</sub> O <sub>4</sub>	LOI
A	42.56	26.76	9.67	1.14	6.59	3.54	0.01	1.26	0.88	7.04	0.11	0.33
B	43.38	18.54	15.29	0.77	12.20	5.43	0.02	1.32	1.05	1.43	0.14	2.12
C	44.51	22.44	15.95	1.19	8.98	1.91	0.05	2.05	0.63	1.55	0.11	28.66
D	39.71	16.38	26.15	0.83	8.86	5.12	0.02	0.87	0.89	0.58	0.35	23.83

**Table 2.** Milling Time and Change of Particle Size and SSA of Fly Ash

Fly Ash	Milling Time (hour)	D10 ( $\mu\text{m}$ )	D50 ( $\mu\text{m}$ )	D90 ( $\mu\text{m}$ )	D100 ( $\mu\text{m}$ )	SSA ( $\text{m}^2/\text{kg}$ )
A	0	1.79	9.15	39.78	142.53	1338
	8	1.77	9.96	34.84	97.87	1286
B	0	1.31	8.55	36.91	162.38	1538
	8	1.91	9.19	33.94	109.47	1324
C	0	8.66	46.76	152.89	660.35	462
	8	1.45	9.01	43.33	141.07	1506
D	0	8.06	42.50	168.00	664.00	521
	8	1.41	9.85	49.10	349.00	1515

**Table 3.** Milling Time and Increase of Specific Gravity of Fly Ash

Fly Ash	Milling Time (hour)	LOI (% by mass)	Specific Gravity	Increase of Specific Gravity
A	0	0.33	2.75	0.44
	8	0.01	3.19	
B	0	2.12	2.06	0.12
	8	0.87	2.18	
C	0	28.66	1.64	0.18
	8	22.71	1.82	
D	0	23.83	1.83	0.77
	8	19.23	2.60	

surface areas (SSA). Apparently, the simple grinding equipment constructed is efficient to grind the coarse material only, it is not so for the fine materials with D90 less than 40  $\mu\text{m}$ . After eight hours milling time, the particle size, the particle size distribution and the specific surface (SSA) area of the four fly ash samples become similar one to the other.

Milling increases the values of the specific gravity of fly ash, although the change does not really show any regular pattern, as can be seen in Table 3. Seemingly, it is due to the change in the particle packing condition to be more dense for the finer particles. On the other hand, the LOI values are slightly reduced after milling, although the values for fly ash C and D are still far above the 6% limit.

### Mixture Composition of HVFA Mortar

For the mixture composition of the mortar specimens, both the water-to-cementitious ratio and sand-to-cementitious ratio were set constant, at 0.2 and 2 by mass, respectively. In this regard, water content was also set constant, as the content of the cementitious materials was also constant. Cementitious materials consisted of the combination of Portland Pozzolan Cement (PPC) and fly ash. PPC is produced by a local cement manufacturer, and the only type available in the market for common use. The use of fly ash in the mixtures was set at 50% by mass, of the total cementitious material, when investigating the effect of milling on the reactivity of different type of fly ash. In evaluating the influence of fly ash usage or replacement ratio, the amount of fly ash was varied from 40 to 60%, by mass, of the total cementitious material.

All the HVFA mortar mixtures were prepared based on a constant consistency condition. Polycarboxylate-based superplasticizer was added to the fresh mixture to obtain the flow diameter of  $18 \pm 1$  cm in mortar flow table test. The constant consistency target is important to ensure that the mortar properties are comparable in term of its compactibility and water-to-cementitious ratio.

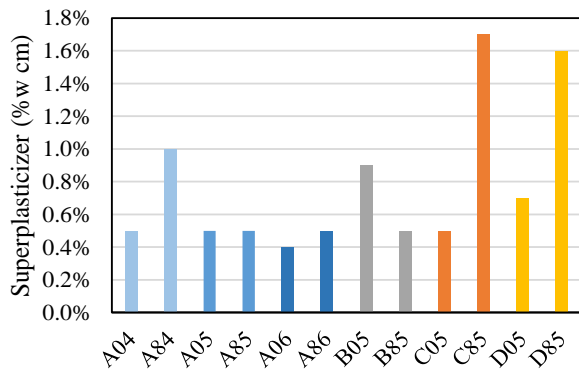
High volume fly ash (HVFA) mortar specimens were casted in cubical shape mould of  $5 \times 5 \times 5$   $\text{cm}^3$ . Curing was performed by soaking the specimens in the fresh water until one day before the testing days at the age of 14 and 28 days. Each compressive strength data represents the average value of the results from testing three cubical specimens.

For the designation, C85 denotes the use of fly ash C with eight hours milling time and 50% fly ash usage of the total cementitious materials, by mass. A06 indicates the use of fly ash A in its original form (0 milling time) and 60% usage of fly ash of the total cementitious materials, by mass.

## Results and Discussion

### Consistency and Superplasticizer Demand

The water content, water-to-cementitious ratio, and sand-to-cementitious ratio of all mortar mixtures were set constant. For the purpose of evaluating the superplasticiser demand when using four different types of fly ash to produce HVFA mortar, the amount of fly ash utilized was set constant as well, at the level of 50% of the total cementitious materials, by mass.



**Figure 3.** Superplasticizer Demand to Achieve Target Mortar Flow Diameter of  $18 \pm 1$  cm

To achieve the similar target consistency for all mixtures, polycarboxylate-based superplasticizer was added, rather than altering the water content. Superplasticizer demand for the target consistency of flow diameter of  $18 \pm 1$  cm, measured from flow table test, is shown in Figure 3.

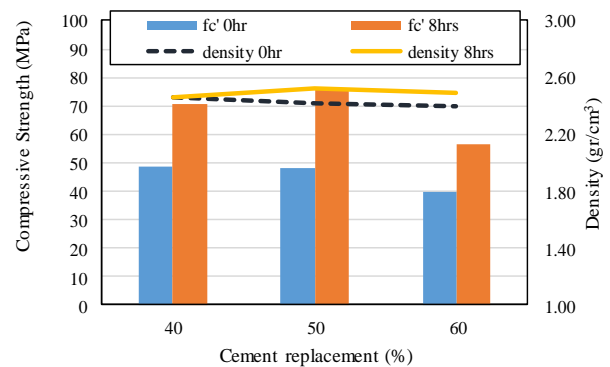
All mortar mixtures utilizing original un-milled fly ash required 0.5-0.8% superplasticizer, by mass of the cementitious materials, to achieve the target flow diameter. For those using the milled ones, in general, fresh mortar mixtures using fly ash A and B did not show any significant change in superplasticizer demand, with the exception of mixture A84, whereby the phenomenon is still un-clear. However, the ones utilizing milled fly ash C and D experienced a significant increase in superplasticizer demand of up to 1.5%.

Most probably this was due to the high carbon content of fly ash C and D that absorbed part of the water and admixture, and lead to higher superplasticizer demand. Milling fly ash increases the number of fly ash particles, and hence also the number of the carbon particles. Milling fly ash also fractures the originally spherical fly ash particles to be particles with irregular shape and rough surface. It lowers the workability of the fresh mortar mix, and thus increases the need for superplasticizer.

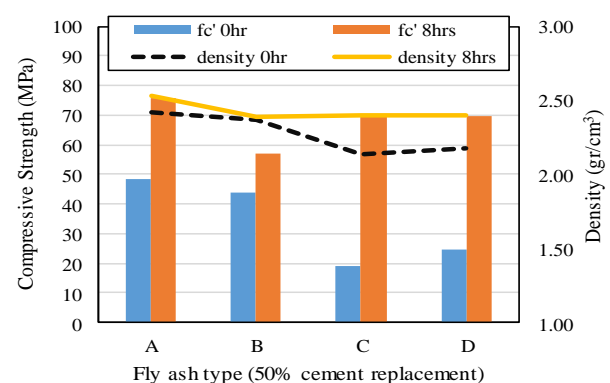
Milling fly ash to reduce its particle size in order to increase its reactivity may affect the workability of mortar or concrete produced. This lack of workability of the fresh mortar or concrete mixtures may cause unexpected results, as the high reduction of strength may happen. This mostly due to difficulty in compaction rather than poor fly ash quality. In this regard, the use of superplasticizer should be considered to control the workability.

### Compressive Strength and Density

Figures 4 and 5 show the 28-day compressive strength and density of HVFA mortar incorporating fly ash A, for both original and milled forms. The amount of fly ash incorporated was varied from 40 to 60%.



**Figure 4.** Compressive Strength and HVFA Mortar Density with Fly Ash A at 28 days



**Figure 5.** Compressive Strength and Density of HVFA Mortar with 50% Replacement Ratio at 28 Days of Age

Milling fine fly ash A for eight hours only slightly affected the particle size of the fly ash. The biggest particle size, as can be seen from the value of D100, reduced from  $142.53 \mu\text{m}$  to  $97.87 \mu\text{m}$ ; while D90 shows only a marginal reduction from  $39.78 \mu\text{m}$  to  $34.84 \mu\text{m}$ . Practically, SSA values remained constant. Apparently, milling equipment used is only effective to grind big fly ash particle, i.e. bigger than  $40 \mu\text{m}$ , whereby its presence in fly ash A is only minor.

Although milling for eight hours only slightly change the particle size distribution of fly ash A, Figure 4 shows that there is a significant increase in compressive strength of HVFA mortar using fly ash A that has been mechanically activated by milling. The amount of fly ash used was varied from 40 to 60% of the total cementitious materials in mass. The measured density of the mortar increased slightly, showing that there could be improvement on the particle packing of the mortar incorporating finer particle size.

Different picture can be seen from Figure 5. HVFA mortar utilizing fine fly ash B gained approximately 20% increase in its strength after using the ground fly ash. However, for the ones using the coarse fly ash C and D, the increase in compressive strength due to using the ground fly ash is very big. HVFA

mortar utilizing ground fly ash C and D gained 264% and 183% strength increase, respectively, compared to those using the untreated fly ash.

The untreated coarse fly ash with high LOIs (i.e. fly ash C and D) are not reactive, as can be seen from the low compressive strength of mortar utilizing them. Milling for eight hours significantly affected the particle size distribution of fly ash C and D. For fly ash C, eight hours milling reduced its D90 size from 152.89 to 43.33  $\mu\text{m}$ , its D50 value from 46.76 to 9.01  $\mu\text{m}$ , and increased the SSA value from 462 to 1506  $\text{m}^2/\text{kg}$ . Very considerable increase also happened to the coarse fly ash D.

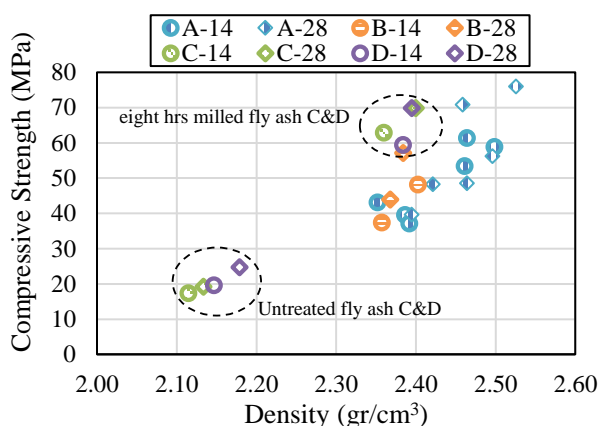
The results confirm that mechanical activation by milling effectively increase the reactivity of coarse fly ash with high LOIs. It also reveal that the simple grinding equipment used is very effective for the purpose.

Low specific gravity of untreated fly ash C and D, i.e. 1.64 and 1.83, respectively, taking more volume in the HVFA mortar, causing lower density for the mortar. Using ground fly ash C and D increased HVFA mortar density.

### LOI, Density, and Mechanical Activation

Relationship between mortar density and its compressive strength is shown in Figure 6. It shows that there are significant improvement on the properties of HVFA mortar incorporating fly ash C and D, compared to the ones using fly ash A and B.

After milling, fly ash C and D with LOIs higher than 20% are eligible to be used as cement replacement material with 50% replacement ratio, to produce comparable compressive strength of HVFA mortar utilizing fly ash A and B. Mechanical activation by means of milling can be the best alternative in an attempt to utilize the undesired coarse fly ash with high LOI in concrete industry.



**Figure 6.** Relationship between the Density and Strength of HVFA Mortar at 14 and 28 days

### Conclusions

1. Simple mechanical activation by using a rod mill improves the reactivity of fly ash significantly, especially the ones with coarse particle size and high LOI, due to change in the particle size distribution.
2. Compressive strength of the HVFA mortar utilizing milled fly ash is significantly higher than the ones utilizing the untreated one. The strength increase is very significant for HVFA mortar incorporating fly ash with high LOI and originally coarse particle size fly ash.
3. Change of the particle shape and size of fly ash affects the rheological behavior of the fresh HVFA mortar mixture. The superplasticizer demand increased when using the milled fly ash to obtain the same consistency.

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